

CROSS SECTION A - A

SETBACK CERTIFICATION REQUIRED - A California licensed surveyor is required to certify the location and setbacks of the new swimming pool prior to beginning excavation. A copy of the certification shall be available to the Building Inspector prior to the first inspection.

NEW CONSTRUCTION REQUIREMENTS. [HSC 115928]
 Whenever a building permit is issued for the construction of a new swimming pool or spa, the pool or spa shall meet all of the following requirements:
 a) (1) The suction outlets of the pool or spa for which the permit is issued shall be equipped to provide circulation throughout the pool or spa as prescribed in paragraphs (2) and (3).
 (2) The swimming pool or spa shall either have at least two circulation suction outlets per pump that shall be hydraulically balanced and symmetrically plumbed through one or more "T" fittings, and that are separated by a distance of at least three feet in any dimension between the suction outlets, or be designed to use alternatives to suction outlets, including, but not limited to, skimmers or perimeter overflow systems to conduct water to the recirculation pump.
 (3) The circulation system shall have the capacity to provide a complete turnover of pool water, as specified in Section 3124B of Chapter 31B of the California Building Standards Code (Title 24 of the California Code of Regulations).
 b) Suction outlets shall be covered with anti-entrapment grates, as specified in the ANSI/APSP-16 performance standard or successor standard designated by the federal Consumer Product Safety Commission, that cannot be removed except with the use of tools. Slots or openings in the grates or similar protective devices shall be of a shape, area, and arrangement that would prevent physical entrapment and would not pose any suction hazard to bathers.
 c) Any backup safety system that an owner of a new swimming pool or spa may choose to install in addition to the requirements set forth in subdivisions (a) and (b) shall meet the standards as published in the document, "Guidelines for Entrapment Hazards: Making Pools and Spas Safer," Publication Number 363, March 2005, United States Consumer Product Safety Commission.

SWIMMING POOL STAGE III DROUGHT RESTRICTIONS (City of Burbank Sustainable Water Use Ordinance, Burbank Municipal Code Title 8, Chapter 2, Article 3):
 All swimming pools, wading pools and spas must be covered with acceptable protection to decrease water evaporation per the Stage III Drought Restrictions. For additional information visit the Burbank Water and Power web site at <https://www.burbankwaterandpower.com/water/water-drought>.

ANTI-ENTRAPMENT COVER REQUIREMENT FOR REMODELS. [HSC 115928.5]
 Whenever a building permit is issued for the remodel or modification of an existing swimming pool, toddler pool, or spa, the permit shall require that the suction outlet or suction outlets of the existing swimming pool, toddler pool, or spa be upgraded so as to be equipped with anti-entrapment grates, as specified in the ANSI/APSP-16 performance standard or a successor standard designated by the federal Consumer Product Safety Commission.

AGREEMENTS TO BUILD; NOTICE OF PROVISIONS. [HSC 115924]
 a) Any person entering into an agreement to build a swimming pool or spa, or to engage in permitted work on a pool or spa covered by this article, shall give the consumer notice of the requirements of this article.
 b) Pursuant to existing law, the Department of Health Services shall have available on the department's Web site, commencing January 1, 2007, approved pool safety information available for consumers to download. Pool contractors are encouraged to share this information with consumers regarding the potential dangers a pool or spa poses to toddlers. Additionally, pool contractors may provide the consumer with swimming pool safety materials produced from organizations such as the United States Consumer Product Safety Commission, Drowning Prevention Foundation, California Coalition for Children's Safety & Health, Safe Kids Worldwide, Association of Pool and Spa Professionals, or the American Academy of Pediatrics.

FINAL INSPECTION
 Prior to the issuance of any final approval for the completion of permitted construction or remodeling work, the Building Division shall inspect the drowning safety prevention devices required by this act, and if no violations are found, shall give final approval. The contractor and/or the owner shall submit the signed Certificate of Compliance to the Building Inspector prior to final inspection.

NOTES

BABYLON POOLS
 556 RIVERDALE DRIVE
 GLENDALE, CA 91204
 LIC# 8556670
 (855) 818-POOL

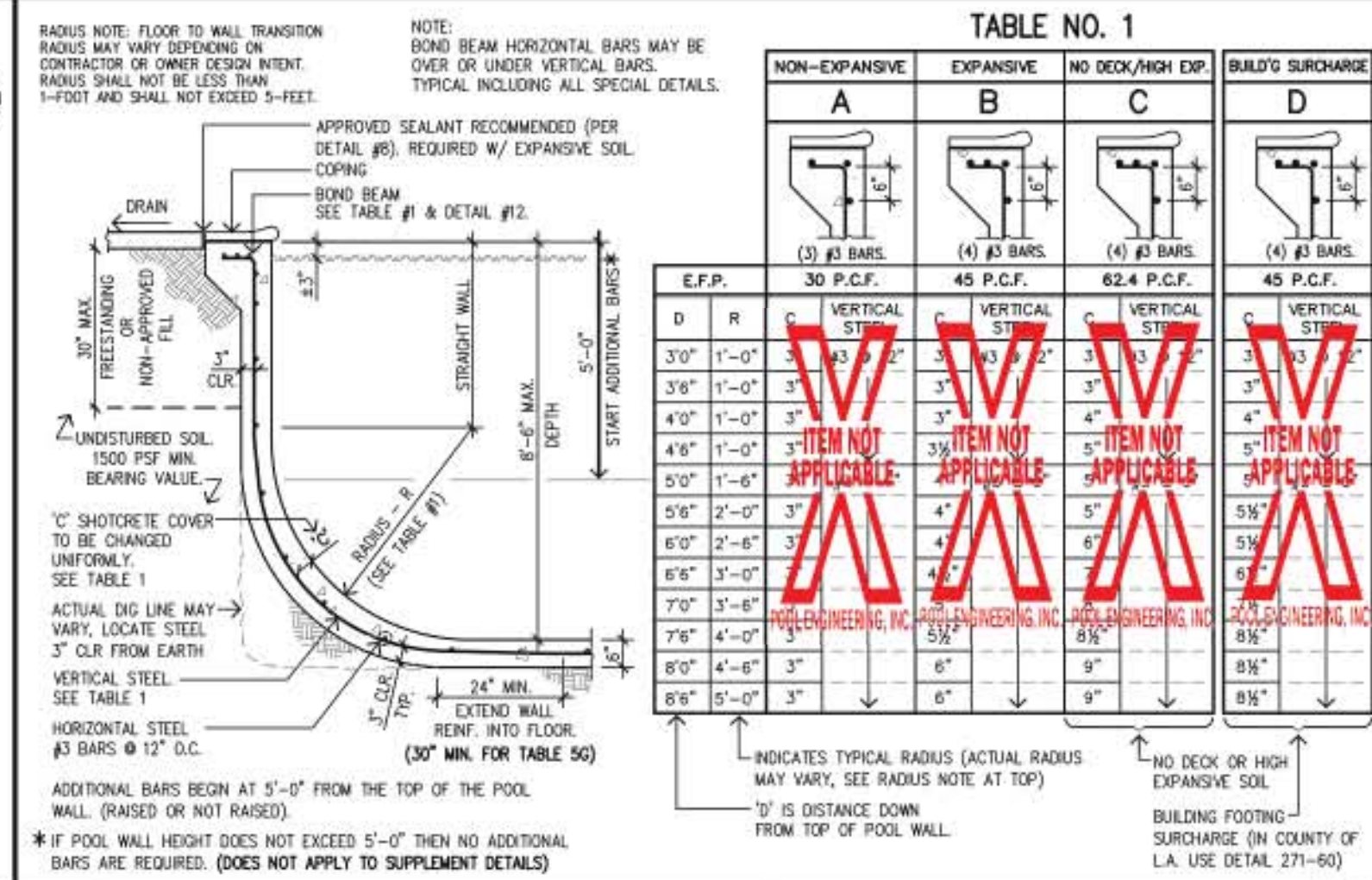
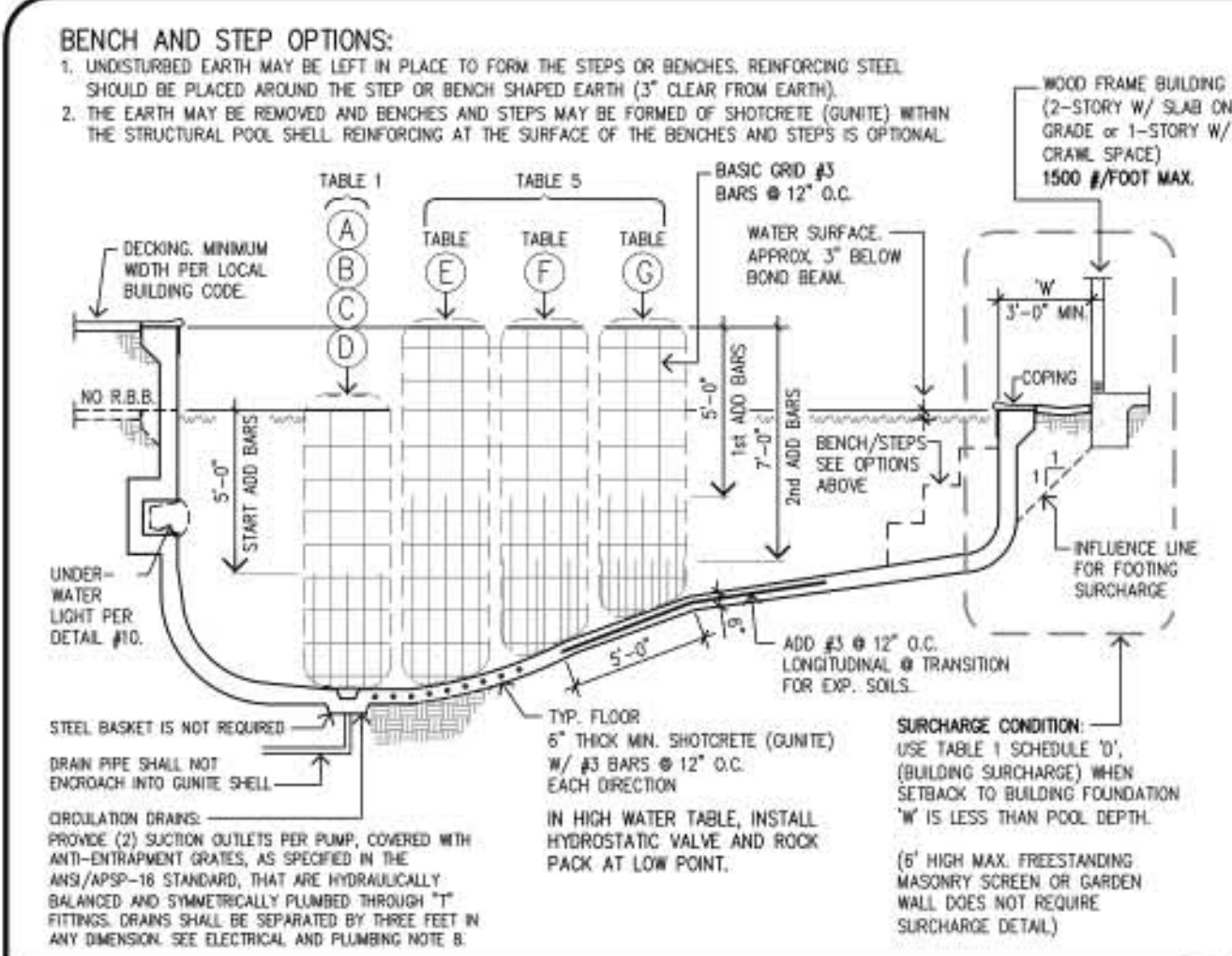
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SITE PLAN

NAREK ISAKHANYAN
 3147 MESA VERDE DRIVE
 BURBANK, CA 91504

| | |
|----------------------|-------------|
| SCALE 1/8" = 1' - 0" | PROJECT NO. |
| DRAWN BY | SHEET NO. |
| CHECKED BY | 2 |
| DATE | |
| DATE OF PRINT | |



GENERAL NOTES

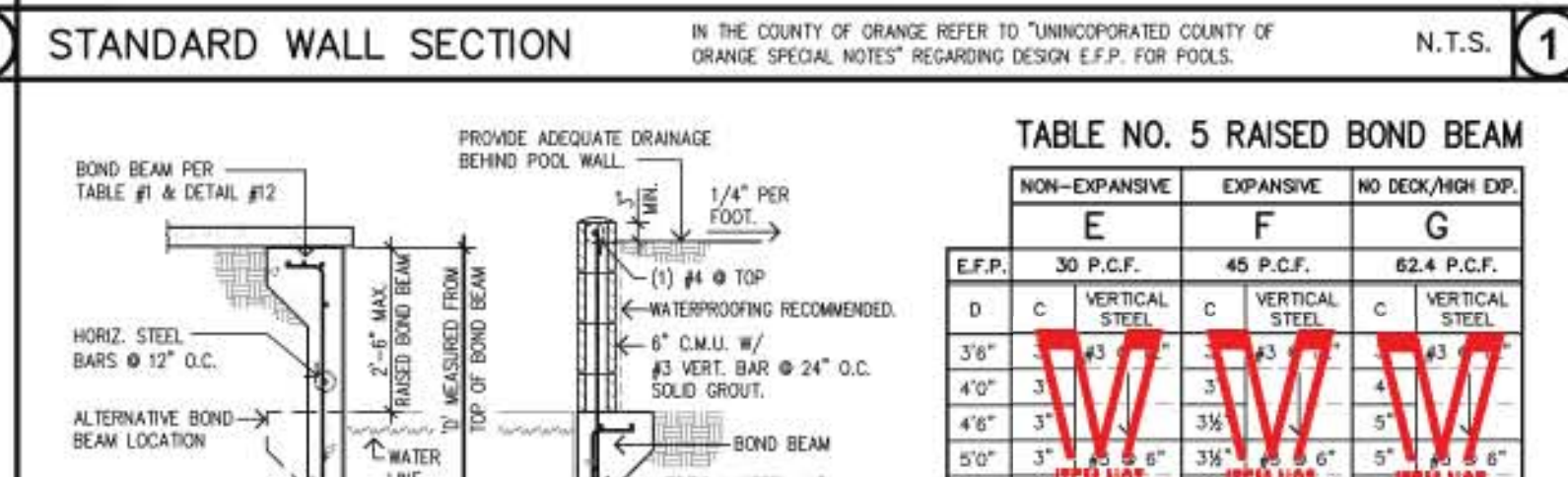
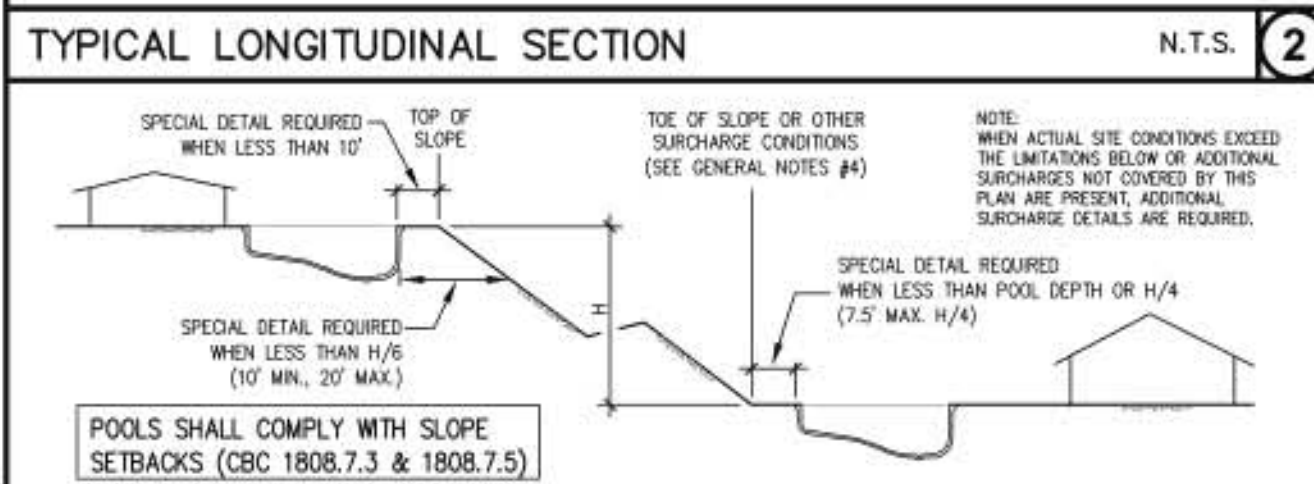
- THIS STANDARD POOL STRUCTURAL PLAN MUST BE ACCOMPANIED BY A CLEAR PLOT PLAN SHOWING POOL AND/OR SPA SHAPE, SETBACK DISTANCE TO PROPERTY LINE, GRADE CHANGES AND SLOPES AND ADJACENT STRUCTURES.
- REPRESENTATIVES OF POOL ENGINEERING INC. HAVE NOT INSPECTED THE SITE & ARE RELYING ON INFORMATION PROVIDED BY THE CONTRACTOR OR OWNER TO DETERMINE THE STANDARD POOL STRUCTURAL PLAN FOR THE ACTUAL SITE CONDITIONS. SHOULD SITE CONDITIONS VARY FROM THAT COVERED BY THIS STANDARD POOL STRUCTURAL PLAN, IT IS THE RESPONSIBILITY OF THE CONTRACTOR OR THE OWNER TO NOTIFY POOL ENGINEERING INC. AND OBTAIN APPLICABLE SPECIAL ENGINEERING DETAILS FOR CONSTRUCTION. THESE SPECIAL DETAILS ARE VALID ONLY FOR STATED EQUIVALENT FLUID PRESSURES AND POOL ENGINEERING INC. RECOMMENDS THAT THE OWNER OR CONTRACTOR OBTAIN A SOILS REPORT.
- POOL ENGINEERING INC. (PEI) RECOMMENDS THAT THE PROPERTY OWNER AND/OR POOL CONTRACTOR OBTAIN A GEOTECHNICAL CONSULTANT TO OBTAIN GEOTECHNICAL RELATED DESIGN CRITERIA FOR THE PROPOSED POOL SITE. IT IS THE RESPONSIBILITY OF THE PROPERTY OWNER AND/OR POOL CONTRACTOR TO REQUIRE THAT THE LICENSED GEOTECHNICAL CONSULTANT CONFIRM THAT THE POOL STRUCTURAL PLAN PROVIDED MEET THE REQUIREMENTS OF THE PROJECT SITE AND THE GEOTECHNICAL REPORT. WHEN A GEOTECHNICAL REPORT HAS NOT BEEN PROVIDED TO PEI IT IS THE OWNER AND/OR CONTRACTOR'S RESPONSIBILITY TO VERIFY THAT THE SITE GEOTECHNICAL CONDITIONS ARE SUITABLE FOR CONSTRUCTION OF THE PROPOSED POOL BASED ON THE PEI PLANS BEING INSTALLED.
- THIS PLAN IS NOT VALID WITHOUT ADDITIONAL SURCHARGE DETAILS WHEN THE CONDITIONS AS SHOWN IN DETAIL #3 APPLY (PER CBC SECTION 1808.7.3). ALL POOLS SHALL COMPLY WITH SLOPE SETBACKS PER CBC SECTION 1808.7.3.
- STANDARD POOL STRUCTURAL PLAN IS NOT INTENDED TO BE APPLICABLE TO NON-STANDARD ITEMS INCLUDING BUT NOT LIMITED TO PLUMBING, ELECTRICAL, FENCING, CONCRETE DECKING AND POOL GEOMETRICS.
- DECKING CONSTRUCTION IS SHOWN AS RECOMMENDED MINIMUM CONSTRUCTION AND DOES NOT DEMONSTRATE A SYSTEM THAT WILL RESIST HEATING DUE TO SOIL EXPANSION.
- ALL CONSTRUCTION SHALL COMPLY WITH THE 2018 EDITIONS OF THE CALIFORNIA BUILDING CODE (CBC), CALIFORNIA ELECTRICAL CODE (CEC), CALIFORNIA MECHANICAL CODE (CMC), CALIFORNIA PLUMBING CODE (CPL), CALIFORNIA ENERGY CODE, 2019 BUILDING ENERGY EFFICIENCY STANDARDS (2019 CALIFORNIA GREEN BUILDING STANDARDS CODE, AND LOCAL ORDINANCES.
- POOLS WITH DIVING BOARDS SHALL MEET DIVING BOARD MANUFACTURER'S POOL GEOMETRIC STANDARDS AND/OR LOCAL CODES.
- SPRINKLER SAFETY EQUIPMENT SHALL BE INSTALLED IN ACCORDANCE WITH LOCAL CODES.
- PUBLIC POOLS REQUIRE COUNTY HEALTH DEPARTMENT APPROVAL AND PROVISIONS FOR ASSISTIVE DEVICES FOR THE DISABLED.
- CONTRACTOR OR OWNER SHALL VERIFY ALL FIELD CONDITIONS & DIMENSIONS AT JOB SITE. POOL LENGTH GRADE BREAK LOCATION IS AS SHOWN ON THE PLAN. THE CONTRACTOR SHALL COMPLY WITH APSP SUGGESTED MINIMUM STANDARDS FOR RESIDENTIAL POOLS OR APPLICABLE STATE AND LOCAL HEALTH DEPARTMENTS REGULATIONS AND MANUFACTURER'S RECOMMENDATIONS.

ELECTRICAL AND PLUMBING

- ALL ELECTRICAL SHALL BE IN CONFORMANCE WITH THE 2019 CEC.
- POOL SHELL AND PERIMETER PAVED AND UNPAVED SURFACES SHALL BE BONDED IN ACCORDANCE WITH CEC 880.26(B). BONDED TO PERIMETER SURFACES SHALL BE PROVIDED AS SPECIFIED IN CEC 880.26(1)(A) OR (2)(B) AND SHALL BE ATTACHED TO THE POOL REINFORCING STEEL OR COPPER CONDUCTOR GRID AT A MINIMUM OF FOUR (4) POINTS UNIFORMLY SPACED AROUND THE PERIMETER OF THE POOL.
- OBTAIN ELECTRICAL AND PLUMBING PERMITS ALONG WITH POOL BUILDING PERMIT.
- ALL EQUIPMENT SHALL BE LISTED AND INSTALLED PER MANUFACTURER'S INSTRUCTIONS AND IN ACCORDANCE WITH LOCAL REGULATIONS.
- POOLS SHALL BE EQUIPPED WITH A FILTERING SYSTEM.
- BACKWASH SHALL BE DISPOSED OF IN AN APPROVED MANNER.
- POOL/SPA WATER HEATER AND GAS PIPING INSTALLATION TO BE IN CONFORMANCE WITH THE CEC.
- SUCTION OUTLETS SHALL BE DESIGNED AND INSTALLED IN ACCORDANCE WITH ANS/APSP-7 AND 2019 CBC SECTION 3109 (HS CODE 88 115920-115929).
- POTENTIAL WATER SUPPLY TO SWIMMING POOLS, SPAS, AND HOT TUBS SHALL BE PROTECTED FROM BACKFLOW IN ACCORDANCE WITH CPC 603.5.20.
- CHEMICAL DISPENSERS SHALL COMPLY WITH 2019 CPC SECTION 603.5.21 REQUIREMENTS FOR PROTECTION AGAINST BACKFLOW AND COMPLY WITH ASSE 1055.

STRUCTURAL NOTES

- SOIL SHALL HAVE A MINIMUM BEARING VALUE OF 1500 PSF. CONCRETE SHALL BE PLACED AGAINST UNDISTURBED SOIL OR BUILDING DEPARTMENT APPROVED 90% COMPACT FILL. THIS PLAN IS NOT SUITABLE WHERE POTENTIAL EXISTS FOR DIFFERENTIAL MOVEMENT FROM DISSIMILAR SOIL CONDITIONS UNDER POOL, INCLUDING BUT NOT LIMITED TO CUT-FILL TRANSITIONS.
- ALL REINFORCING STEEL SHALL BE DEFORMED BARS & CONFORM TO ASTM A615 GRADE 40 FOR #3 BARS AND #4 BARS. SPLICES TO BE LAPPED A MINIMUM OF 24". MINIMUM CLEARANCE BETWEEN PARALLEL BARS IS 2 1/2".
- (1) #4 BAR IS EQUIVALENT TO AND MAY BE USED IN PLACE OF (2) #3 BARS, WITH THE EXCEPTION THAT IF #4 BARS ARE USED FOR THE BASIC GRID, THE MAXIMUM SPACING IS #4 BARS AT 18" O.C.
- THE PLAN TABLES SPECIFY THE MINIMUM REINFORCEMENT REQUIRED FOR CONVENIENCE OF THE INSTALLER, THERE MAY BE MORE REINFORCEMENT THAN SPECIFIED AT ANY GIVEN POINT IN THE POOL STRUCTURE.
- BONDING/GROUNDING (PER THE CEC) OF THE STRUCTURAL REINFORCING MUST BE INSTALLED PRIOR TO PLACEMENT OF CONCRETE.
- SHOTCRETE (GUNITE) TO BE IN CONFORMANCE WITH CBC SECTION 1908 & SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS, WHERE APPLICABLE. SHOTCRETE (GUNITE) TO BE IN CONFORMANCE WITH CBC SECTION 1904 DURABILITY REQUIREMENTS. AGGREGATE SHALL NOT EXCEED 3/4"-INCH.
- CEMENT SHALL CONFORM TO CBC SECTION 1903.1, AD 318 CHAP. 19 & ASTM C 150.
- SHOTCRETE/GUNITE IN CONTACT WITH SOIL SHALL BE IN ACCORDANCE WITH AD 318 SECTION 19.3 FOR CONCRETE EXPOSURE TO SULFATE AND AS DIRECTED BY LOCAL BUILDING OFFICIAL.
- KEEP CONCRETE DAMP CONTINUOUSLY FOR 14 DAYS.
- ALL INTERIOR SURFACES OF POOL/SPA SHALL BE COATED WITH A WATERPROOF SURFACE.



GLAZING IN HAZARDOUS LOCATIONS

WHEN REQUIRED BY THE BUILDING OFFICIAL, GLAZING SHALL COMPLY WITH THE CBC SECTION 2406.4, INCLUDING LOCALLY ADOPTED AMENDMENTS.

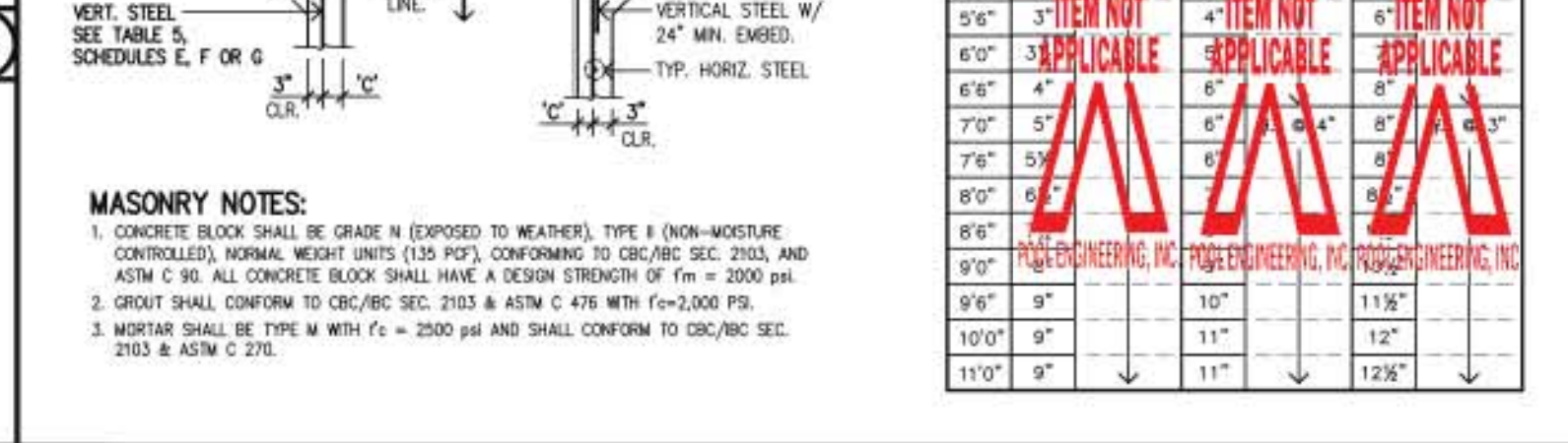
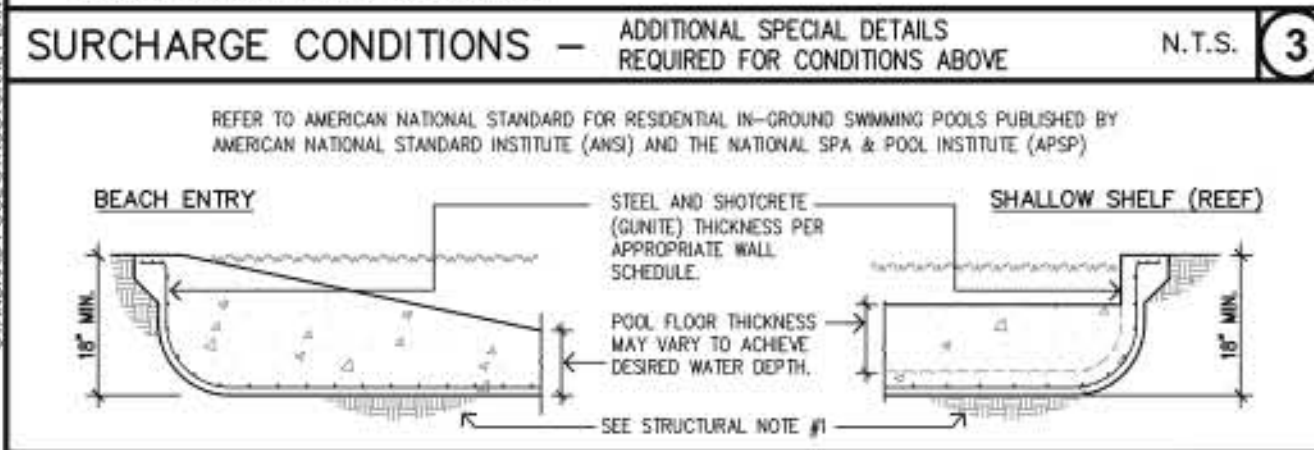
- GLAZING IN WALLS AND FENCES USED AS A BARRIER SHALL BE SAFETY GLAZING WHEN ALL OF THE FOLLOWING CONDITIONS ARE PRESENT:
 - THE BOTTOM EDGE OF THE GLAZING IS LESS THAN 60" ABOVE ANY STANDING OR WALKING SURFACE.
 - THE GLAZING IS WITHIN 5 FEET OF A SWIMMING POOL OR SPA DECK AREA.

CONTINUOUS SHOTCRETE INSPECTION

WHERE REQUIRED BY THE PERMITTING AGENCY, PNEUMATIC CONCRETE PLACEMENT SHALL BE INSPECTED BY A SPECIALIST IN CONFORMANCE WITH CBC SECTION 1704, WHO SHALL SUBMIT A STATEMENT INDICATING COMPLIANCE WITH THE PLANS AND SPECIFICATIONS.

TABLE NO. 5 RAISED BOND BEAM

| | | NON-EXPANSIVE | EXPANSIVE | NO DECK/HIGH EXP. | G |
|-------|----|----------------|----------------|-------------------|----------------|
| | | E.F.P. | 30 P.C.F. | 45 P.C.F. | 62.4 P.C.F. |
| D | C | VERTICAL STEEL | VERTICAL STEEL | VERTICAL STEEL | VERTICAL STEEL |
| 3'6" | 3" | 3" | 3" | 3" | 3" |
| 4'0" | 3" | 3" | 3" | 3" | 3" |
| 4'6" | 3" | 3" | 3" | 3" | 3" |
| 5'0" | 3" | 3" | 3" | 3" | 3" |
| 5'6" | 3" | 3" | 3" | 3" | 3" |
| 6'0" | 3" | 3" | 3" | 3" | 3" |
| 6'6" | 3" | 3" | 3" | 3" | 3" |
| 7'0" | 3" | 3" | 3" | 3" | 3" |
| 7'6" | 3" | 3" | 3" | 3" | 3" |
| 8'0" | 3" | 3" | 3" | 3" | 3" |
| 8'6" | 3" | 3" | 3" | 3" | 3" |
| 9'0" | 3" | 3" | 3" | 3" | 3" |
| 9'6" | 3" | 3" | 3" | 3" | 3" |
| 10'0" | 3" | 3" | 3" | 3" | 3" |
| 11'0" | 3" | 3" | 3" | 3" | 3" |



COUNTY OF LOS ANGELES SPECIAL NOTES

- IN THE COUNTY OF LOS ANGELES, THE DEPARTMENT OF PUBLIC WORKS, BUILDING & SAFETY/LAND DEVELOPMENT DIVISION NOTES TITLED BARRIERS FOR SWIMMING POOL, SPAS, AND HOT TUBS FOR R-3 OCCUPANCIES SHALL BE ATTACHED TO AND MADE A PART OF THE PLAN.
- CONSTRUCTION TO BE PER THE LATEST ADOPTED EDITION OF THE LOS ANGELES COUNTY BUILDING CODE (LABC).
- IN LOS ANGELES COUNTIES SETBACK TO TOE OF ASCENDING SLOPE SHALL NOT BE LESS THAN 1 1/2 FEET & SETBACK TO TOP OF DESCENDING SLOPE SHALL NOT BE LESS THAN 2 1/2 FEET.
- ALL POOL CONSTRUCTION SHALL BE BASED ON AN EQUIVALENT FLUID PRESSURE EQUAL TO OR GREATER THAN 60 P.C.F., UNLESS A SOILS REPORT IS SUBMITTED FOR REVIEW.

CITY OF SIMI VALLEY SPECIAL NOTES

- SEE THE ATTACHED "CITY OF SIMI VALLEY RESIDENTIAL SWIMMING POOL NOTES" FOR BUILDING CODE REQUIREMENTS AND DISPOSAL OF POOL WASTEWATER.
- THE MINIMUM DESIGN EQUIVALENT FLUID PRESSURE (EFP) IS 60 PSF/FT (POF) FOR POOLS SET IN LEVEL GRADE/LEVEL BACKFILL THAT MEET THE CITY OF SIMI VALLEY SLOPE SETBACK REQUIREMENTS FOR POOL PROJECTS THAT DO NOT SUBMIT OR REQUIRE A SOILS REPORT (CBC TABLE 1810.1).

UNINCORPORATED COUNTY OF ORANGE SPECIAL NOTES

- WHERE SITE SPECIFIC SOIL TESTING HAS NOT BEEN PERFORMED:
 - POOLS SHALL BE DESIGNED FOR A MINIMUM DESIGN EQUIVALENT FLUID PRESSURE (E.F.P.) OF 100 P.S.F.
 - CONCRETE IN CONTACT WITH SOIL SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH (F_c) EQUAL TO 4500 P.S.I., TYPE V CEMENT, AND W/C RATIO EQUAL TO 0.45. (THIS SHALL SUPERSEDE ALL OTHER LOWER F_c SPECIFICATIONS SET FORTH).
 - DROWNING PREVENTION SAFETY FEATURES SHALL BE PROVIDED PER COUNTY ORDINANCE NO. 19-006 SECTION 7-1-54 SECTION AVOID.

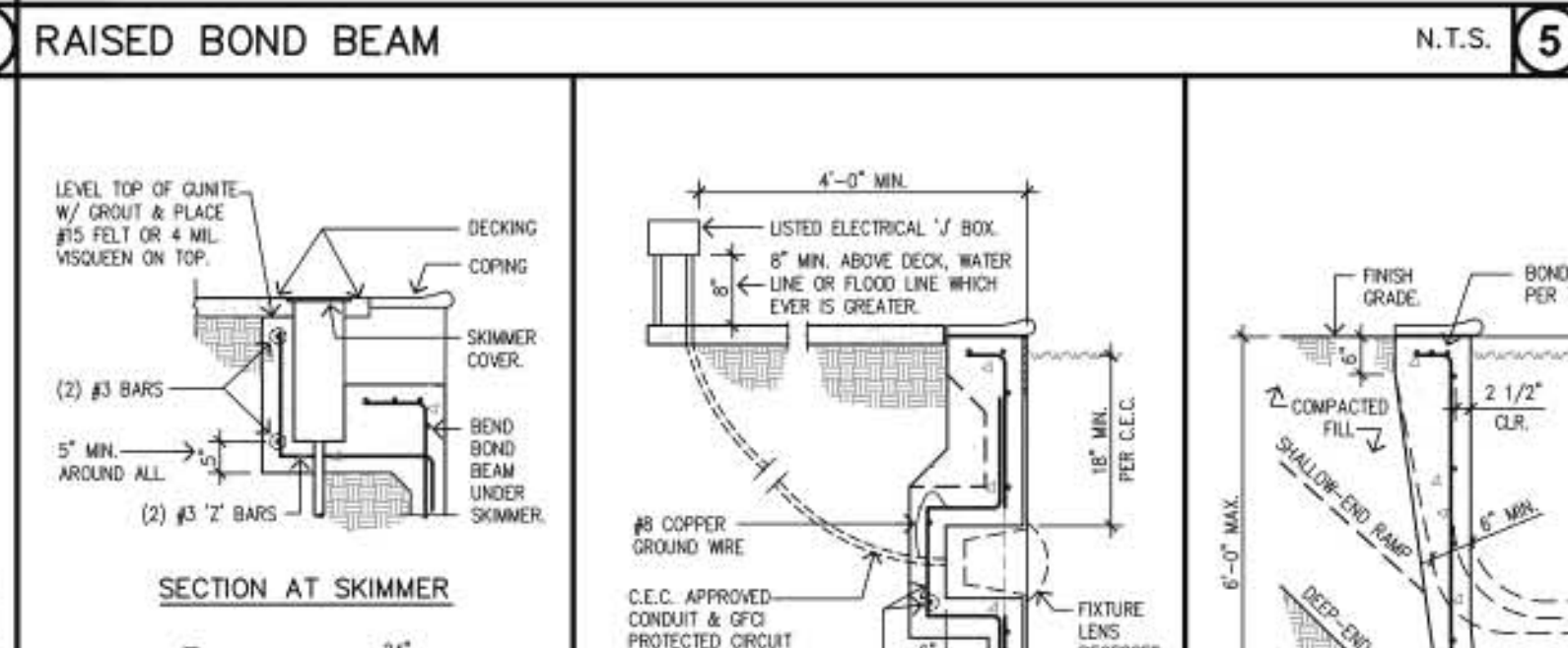
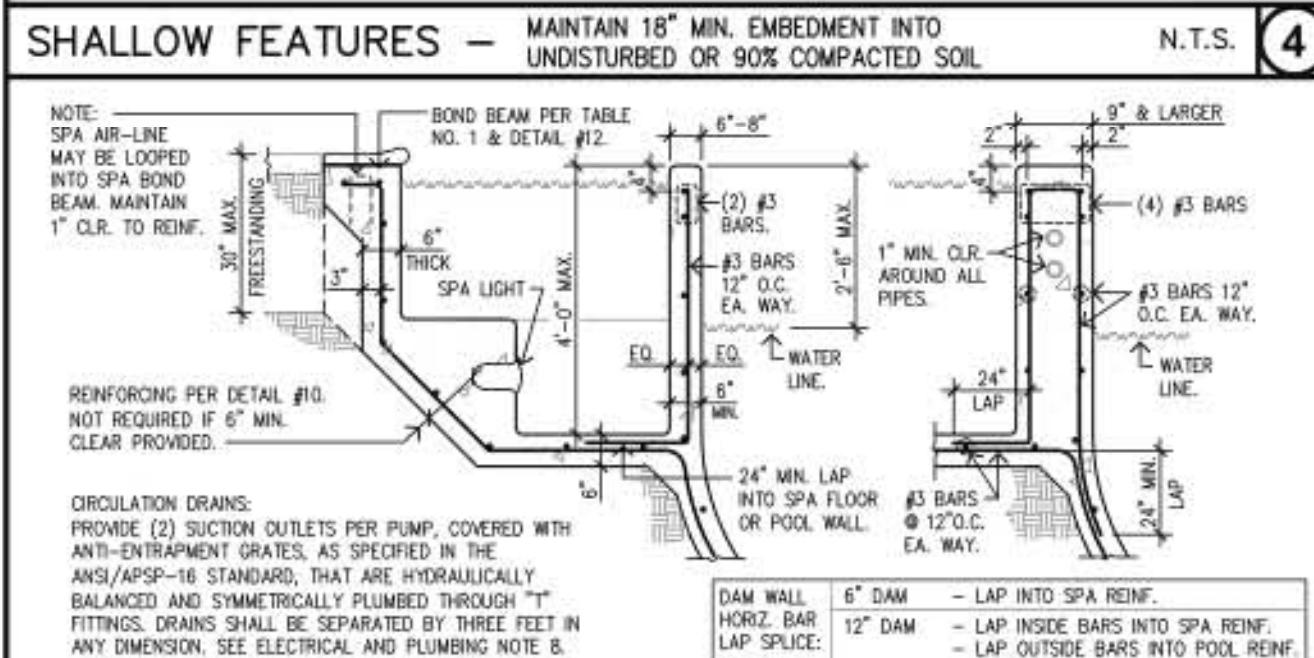
CITY OF SANTA CLARITA SPECIAL NOTES

CONSTRUCTION MATERIALS, CONCRETE, PLASTER, GROUT, SAND, AND GROUTS AS WELL AS ANY WASH WATERS ARE PROHIBITED FROM ENTERING THE STREET, GUTTER, AND STORM DRAIN SYSTEM PER THE SANTA CLARITA MUNICIPAL CODE.

USE PROPER BEST MANAGEMENT PRACTICES TO PREVENT OR CAPTURE ANY CONSTRUCTION ACTIVITY RUNOFF INTO THE STORM DRAIN SYSTEM.

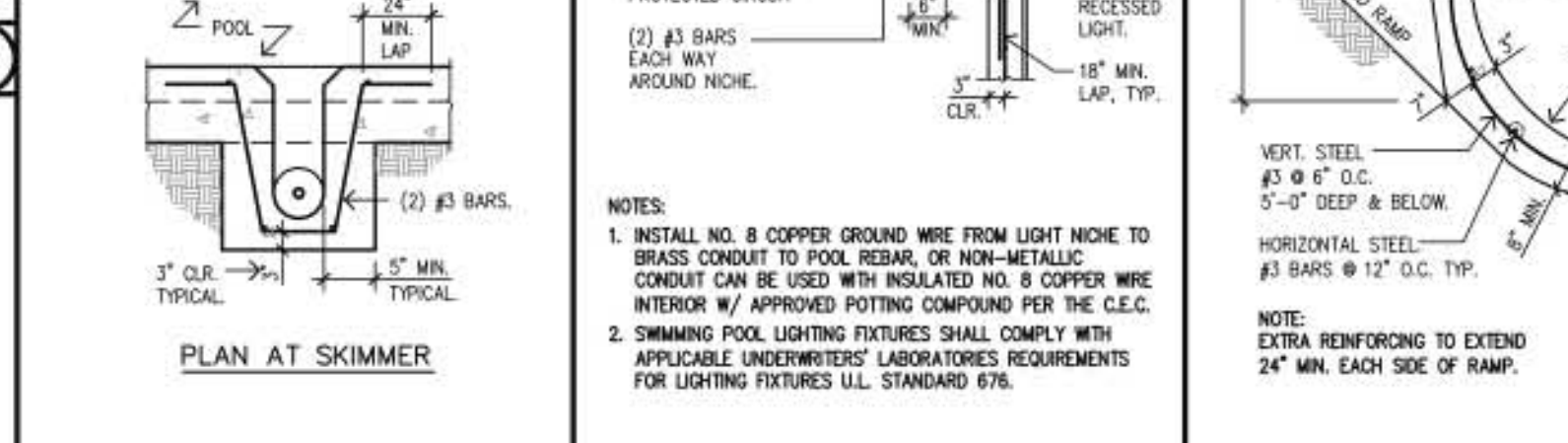
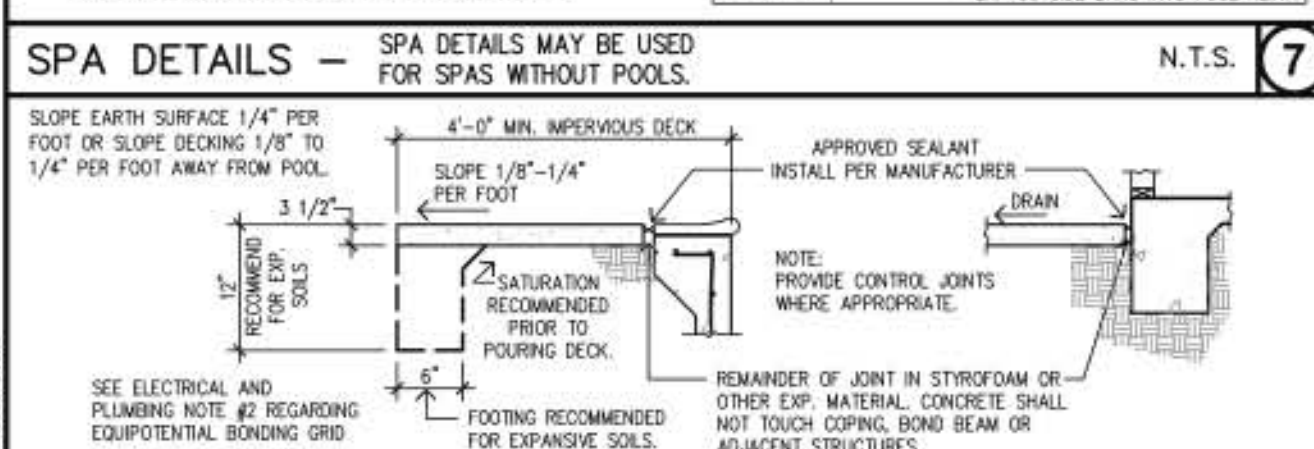
COUNTY OF SAN BERNARDINO SPECIAL NOTES

- THESE PLANS ARE FOR PRIVATE RESIDENTIAL USE ONLY.
- ALL POOL CONSTRUCTION SHALL COMPLY WITH THE 2018 INTERNATIONAL SWIMMING POOL AND SPA CODE (ISPSOC) AND COUNTY OF SAN BERNARDINO INFORMATION BULLETIN 18-001. PERMITS/PLANS SHALL PROVIDE PLANS SHOWING POOL ARCHITECTURAL DESIGN AND GEOMETRY.
- IN THE ABSENCE OF SITE SPECIFIC SOIL TESTING, POOL CONSTRUCTION SHALL BE BASED ON AN E.F.P. OF 60 P.C.F. OR GREATER AND ALL CONCRETE IN CONTACT WITH SOIL SHALL HAVE AT MIN: F_c = 4500 PSI, TYPE V CEMENT AND W/C RATIO = 0.45.
- ENGINEER OF RECORD SHALL PROVIDE SITE PLAN REVIEW STAMP AND SIGNATURE ON SITE PLAN VERIFYING APPROPRIATE PROPOSED POOL LOCATION.



NOTES

BY THE USE OF THIS PLAN, THE USER ACKNOWLEDGES THAT HE HAS READ & UNDERSTANDS ALL OF THE NOTES INCLUDED HEREIN.



FOR USE ONLY AT 3147 Mesa Verde Dr Burbank CA 91504

21-03788

Date: 3/15/2021

PLAN VALID ONLY WITH WET STAMP & ENGINEER'S SIGNATURE IN RED INK ON PLAN.

AUTHORIZED SIGNATURES:
 RONALD L. LACHER, P.E.
 TODD L. LACHER, P.E.
 CHRIS BIEDENBACH, P.E.
 MATTHEW THOMPSON, P.E.

EXPANSIVE SOIL DETAILS

IN EXPANSIVE SOILS WHERE MIN DECK REQS ARE NOT MET USE TABLE 1, SCHEDULE 'C'



BOND BEAM DETAILS

CALCULATIONS

METHODOLOGY:

γ = EQUIVALENT FLUID PRESSURE

CASE I
 $OTM = 1/6 \gamma H^3$ WHERE $\gamma = 85$ pcf
 NET MOM = OTM - RESISTING MOMENT

CASE II
 $OTM = 1/6 \gamma H^3$ WHERE $\gamma = 62.4$ pcf
 NET MOM = OTM + RESISTING MOMENT

$$f_s = \frac{M(12 \text{ in/ft})}{A_s j d} = \frac{M(12)}{A_s (0.887) d}$$

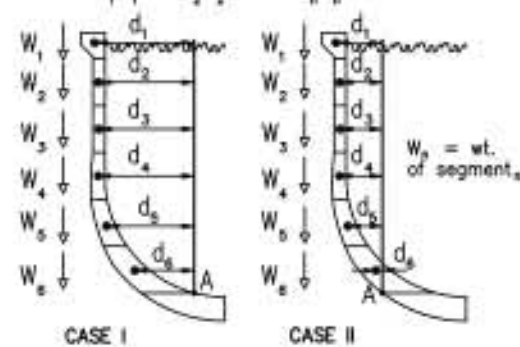
$$f_c = \frac{M(2) 12 \text{ in/ft}}{j k b d^2} = \frac{M(2)(12)}{(0.887)(0.339)(12) d^2} < 1125 \text{ psi}$$

$$s_c = \frac{(1/2) \gamma H^3}{(12 \text{ in/ft}) j d} = \frac{\gamma H^3}{(2)(12)(0.887) d} < 55 \text{ psi}$$

$f'_c = 2,500$ psi
 $F_s = 20,000$ psi
 $f_c = 0.45 f'_c = 1125$ psi
 $V_c = 1.1 \sqrt{f'_c} = 55$ psi

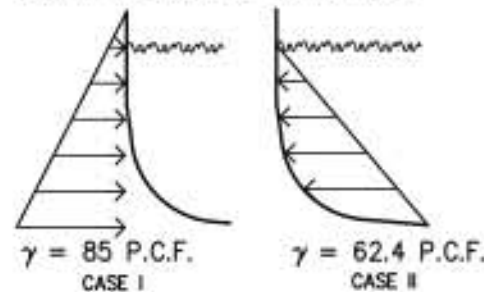
RESISTING MOMENT:

RESISTING MOMENT ABOUT POINT A
 $RM = W_1 d_1 + W_2 d_2 + \dots + W_n d_n$



LOADING DIAGRAM:

THIS DETAIL IS DESIGNED FOR EACH OF THE LOAD CASES DEFINED BELOW.



CALCULATION RESULTS:

3'-0" to 4'-0" RAISED BOND BEAM
 FREESTANDING WALL
 EQUIVALENT FLUID PRESSURE = 85 P.C.F.
 RESULTS FOR 4'-0" RAISED BOND BEAM

| HEIGHT 'H' | SOIL OTM ft-# | WATER OTM ft-# | SOIL RM ft-# | WATER RM ft-# | NET MOM | CASE I d1 SOIL | CASE II d2 WATER | VERTICAL STEEL | f _s p.s.i. | f _c p.s.i. | s _c p.s.i. |
|------------|---------------|----------------|--------------|---------------|---------|----------------|------------------|----------------|-----------------------|-----------------------|-----------------------|
| 2'-0" | 113 | 0 | 38 | -38 | 76 | 3" | 3" | #3 @ 12" | 2964 | 82 | 4.7 |
| 2'-6" | 221 | 0 | 47 | -47 | 174 | 3" | 3" | #3 @ 6" | 3508 | 145 | 7.4 |
| 3'-6" | 607 | 0 | 66 | -66 | 542 | 3" | 3" | " | 10886 | 451 | 14.5 |
| 4'-6" | 1291 | 1 | 84 | -84 | 1207 | 3" | 3" | #3 @ 3" | 12558 | 790 | 23.9 |
| 5'-6" | 2357 | 35 | 105 | -138 | 2252 | 4" | 3" | " | 17308 | 913 | 26.8 |
| 6'-6" | 3891 | 163 | 133 | -184 | 3757 | 4 1/2" | 3" | add 3 #4 | 16517 | 1074 | 33.3 |
| 7'-6" | 5977 | 446 | 170 | -290 | 5807 | 6" | 3" | " | 18843 | 1027 | 33.2 |
| 8'-6" | 8700 | 948 | 290 | -378 | 8410 | 8" | 3" | add 3 #5 | 13964 | 809 | 32.0 |
| 9'-6" | 12146 | 1730 | 624 | -353 | 11522 | 10" | 3" | " | 15126 | 765 | 32.0 |
| 10'-6" | 16400 | 2856 | 1309 | -119 | 15091 | 10 1/2" | 4" | #5 @ 3" | 15952 | 870 | 37.2 |
| 11'-6" | 21546 | 4388 | 2676 | 347 | 18870 | 11 1/2" | 5" | " | 18126 | 956 | 40.7 |
| 12'-6" | 27689 | 6387 | 8574 | -295 | 19095 | 11 1/2" | 6 1/2" | " | 18343 | 947 | 48.1 |

THIS DETAIL TO BE USED WHEN EITHER OF THE FOLLOWING (2) CONDITIONS EXIST:
 1. 'A' IS LESS THAN 10'-0".
 2. ALTERNATE CONDITION '2'.

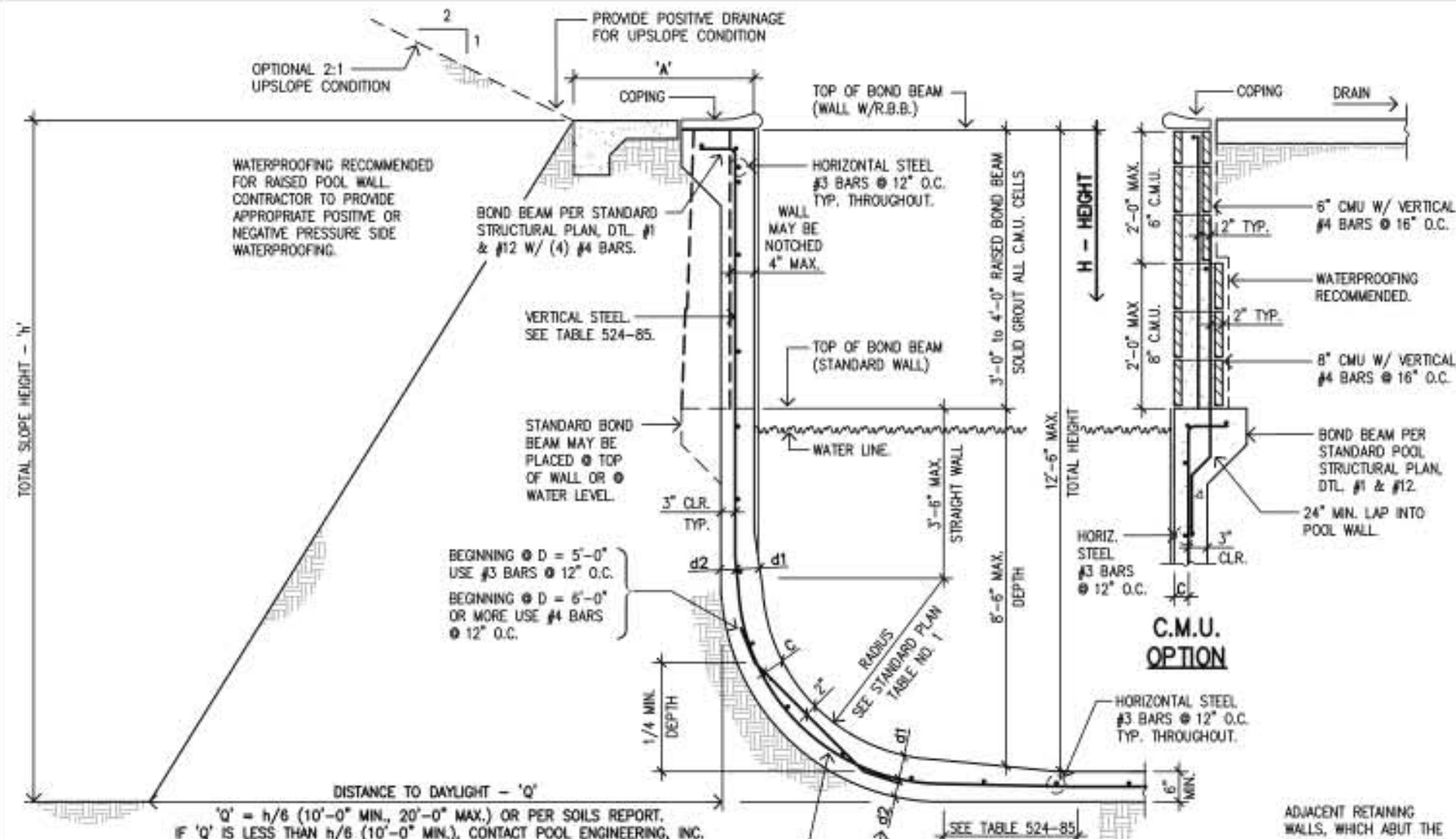
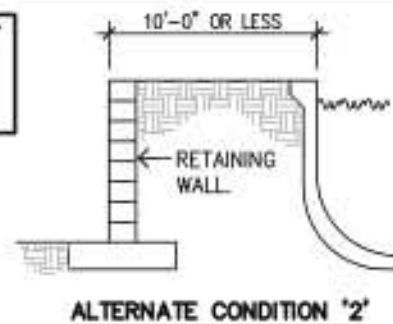


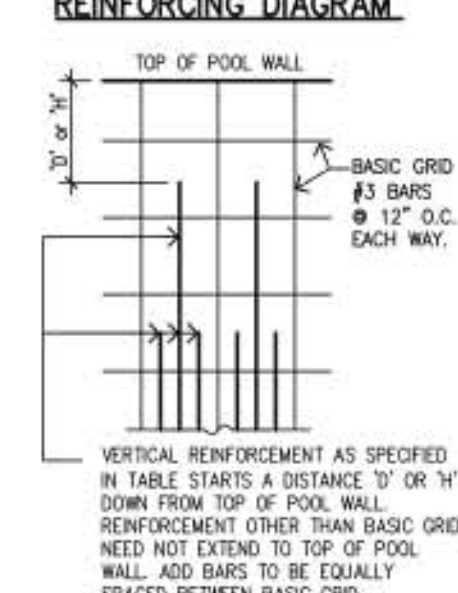
TABLE 524-85

'D' OR 'H' IS DISTANCE FROM TOP OF POOL WALL DOWNWARD. BEGIN SPECIFIED STEEL & GUNITE THICKNESS AT INDICATED 'D' OR 'H' DEPTH (SEE STANDARD STRUCTURAL PLAN, DETAIL #2).

| TOTAL HEIGHT | 4'-0" MAX. RAISED BOND BEAM | | | REQ'D TRANS. |
|--------------|-----------------------------|--------|----------------|--------------|
| H | d1 | d2 | VERTICAL STEEL | |
| 0 to 2'-0" | 3" | 3" | #3 @ 12" | 2'-0" |
| 2'-6" | 3" | 3" | #3 @ 6" | 2'-0" |
| 3'-6" | 3" | 3" | " | 2'-0" |
| 4'-6" | 3" | 3" | #3 @ 3" | 2'-0" |
| 5'-0" | 3" | 3" | " | 2'-0" |
| 5'-6" | 4" | 3" | " | 2'-4" |
| 6'-0" | 4 1/2" | 3" | " | 3'-0" |
| 6'-6" | 4 1/2" | 3" | add 3 #4 | 3'-8" |
| 7'-0" | 5 1/2" | 3" | " | 4'-3" |
| 7'-6" | 6" | 3" | " | 4'-10" |
| 8'-6" | 8" | 3" | add 3 #5 | 5'-11" |
| 9'-6" | 10" | 3" | " | 6'-0" |
| 10'-0" | 10" | 3 1/2" | " | 6'-3" |
| 10'-6" | 10 1/2" | 4" | #5 @ 3" | 6'-4" |
| 11'-6" | 11 1/2" | 5" | " | 6'-6" |
| 12'-0" | 11 1/2" | 6" | " | 6'-6" |
| 12'-6" | 11 1/2" | 6 1/2" | " | 6'-8" |

| TOTAL HEIGHT | 3'-0" MAX. RAISED BOND BEAM | | | REQ'D TRANS. |
|--------------|-----------------------------|----|----------------|--------------|
| H | d1 | d2 | VERTICAL STEEL | |
| 0 to 2'-0" | 3" | 3" | #3 @ 12" | 2'-0" |
| 2'-6" | 3" | 3" | #3 @ 6" | 2'-0" |
| 3'-6" | 3" | 3" | " | 2'-0" |
| 4'-6" | 3" | 3" | #3 @ 3" | 2'-0" |
| 5'-0" | 3" | 3" | " | 2'-0" |
| 5'-6" | 4" | 3" | " | 2'-4" |
| 6'-0" | 4 1/2" | 3" | " | 3'-0" |
| 6'-6" | 4 1/2" | 3" | add 3 #4 | 3'-8" |
| 7'-6" | 6 1/2" | 3" | " | 4'-10" |
| 9'-6" | 9 1/2" | 4" | " | 5'-3" |
| 10'-6" | 10 1/2" | 5" | " | 5'-5" |
| 11'-0" | 10 1/2" | 6" | " | 5'-6" |
| 11'-6" | 10 1/2" | 6" | " | 5'-8" |

TYPICAL ADD BAR REINFORCING DIAGRAM



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Date: 3/15/2021

21-03789

FREESTANDING WALL
 3'-0" to 4'-0" RAISED BOND BEAM
 EQUIVALENT FLUID PRESSURE = 85 P.C.F.

DETAIL #524-85

PLAN VALID ONLY WITH ENGINEER'S SIGNATURE IN RED INK ON PLAN.
 THIS DETAIL TO BE USED IN CONJUNCTION WITH STANDARD POOL STRUCTURAL PLAN

CALCULATIONS

METHODOLOGY:

γ = EQUIVALENT FLUID PRESSURE
CASE I
 (SURCHARGE LOADING BASED ON BOUSSINESQ METHOD, MODIFIED BY TERZAGI FOR TYPICAL BUILDING/FOOTING, 1,500 P.S.F. BEARING PRESSURE).

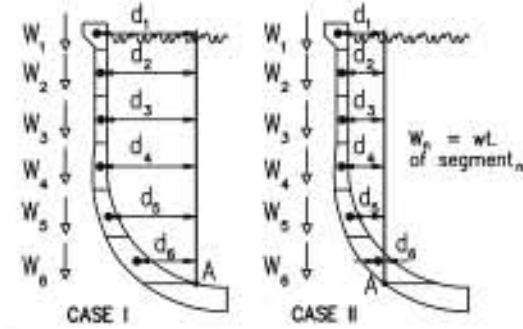
γ = EQUIVALENT FLUID PRESSURE
 $OTM = 1/6 \gamma H^3 + \sum [(P_i)(x_i)]$
 WHERE $\gamma = 70$ p.c.f. AND
 $P_i = 1/2(\sigma_i + \sigma_{i+1}) \times 6$ in
 r_i = vertical dist. from P_i to z depth.
 NET MOM = OTM - RESISTING MOMENT
 $f_s = \frac{M(12 \text{ in/ft})}{A_s j d} = \frac{M_t (12)}{A_s (0.887) d}$

$f_c = \frac{M(2) 12 \text{ in/ft}}{j k b d^2} = \frac{M_t (2)(12)}{(0.887)(0.339)(12) d^2} < 1125 \text{ psi}$
 $\nu_c = \frac{(1/2) \gamma H^2}{(12 \text{ in/ft}) j d} = \frac{\gamma H^2}{(2)(12)(0.887) d} < 55 \text{ psi}$
 $f'_c = 2,500 \text{ p.s.i.}$
 $F_s = 20,000 \text{ p.s.i.}$
 $f_c = 0.45 f'_c = 1125 \text{ p.s.i.}$
 $\nu_c = 1.1 \sqrt{f'_c} = 55 \text{ p.s.i.}$

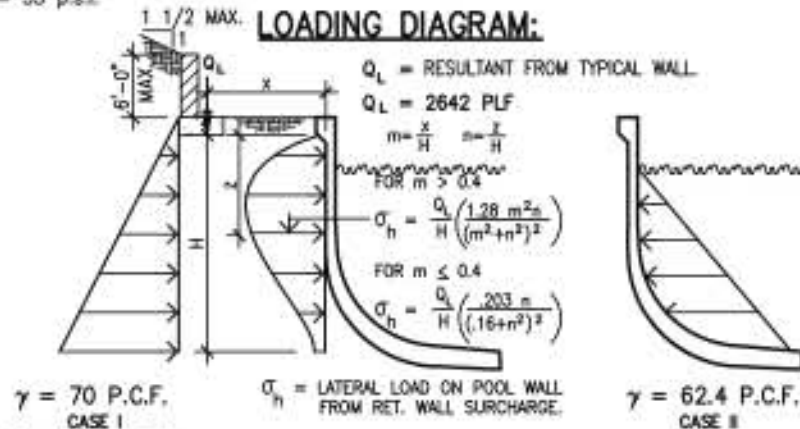
γ = EQUIVALENT FLUID PRESSURE
CASE II
 $OTM = 1/6 \gamma H^3$ WHERE $\gamma = 62.4$ pcf
 NET MOM = OTM + RESISTING MOMENT
 $f_s = \frac{M(12 \text{ in/ft})}{A_s j d} = \frac{M_t (12)}{A_s (0.887) d}$
 $f_c = \frac{M(2) 12 \text{ in/ft}}{j k b d^2} = \frac{M_t (2)(12)}{(0.887)(0.339)(12) d^2} < 1125 \text{ psi}$
 $\nu_c = \frac{(1/2) \gamma H^2}{(12 \text{ in/ft}) j d} = \frac{\gamma H^2}{(2)(12)(0.887) d} < 55 \text{ psi}$
 $f'_c = 2,500 \text{ psi}$ $f_c = 0.45 f'_c = 1125 \text{ psi}$
 $F_s = 20,000 \text{ psi}$ $\nu_c = 1.1 \sqrt{f'_c} = 55 \text{ psi}$

RESISTING MOMENT:

RESISTING MOMENT ABOUT POINT A
 $RM = W_1 d_1 + W_2 d_2 + \dots + W_n d_n$



LOADING DIAGRAM:



CALCULATION RESULTS: FREESTANDING WALL WITH RETAINING WALL SURCHARGE
 EQUIVALENT FLUID PRESSURE = 70 P.C.F.
 RESULTS FOR 2'-6" MAX. RAISED BOND BEAM

| HEIGHT 'H' | SOIL OTM ft-# | LOAD OTM ft-# | WATER OTM ft-# | SOIL RM ft-# | WATER RM ft-# | NET MOM | CASE I d1 SOIL | CASE II d2 WATER | VERTICAL STEEL | f _s p.s.i. | f _c p.s.i. | ν _c p.s.i. |
|------------|---------------|---------------|----------------|--------------|---------------|---------|----------------|------------------|----------------|-----------------------|-----------------------|-----------------------|
| 1'-6" | 39 | 0 | 0 | 69 | -16 | -16 | 3" | 3" | #3 @ 12" | 611 | 17 | 2.9 |
| 2'-0" | 93 | 33 | 0 | 78 | -25 | 48 | 3" | 3" | #3 @ 6" | 961 | 40 | 6.5 |
| 3'-0" | 315 | 237 | 1 | 97 | -44 | 455 | 3 1/2" | 3" | " | 7785 | 295 | 15.3 |
| 4'-0" | 747 | 696 | 35 | 119 | -123 | 1323 | 5" | 3" | #3 @ 3" | 8045 | 371 | 19.2 |
| 5'-0" | 1458 | 1404 | 163 | 161 | -234 | 2701 | 7" | 3" | " | 11553 | 437 | 20.2 |
| 6'-0" | 2520 | 2320 | 446 | 226 | -362 | 4613 | 8" | 3 1/2" | odd 3 #4 | 11066 | 507 | 23.6 |
| 7'-0" | 4002 | 3393 | 948 | 373 | -468 | 7021 | 8 1/2" | 5" | " | 15805 | 699 | 28.0 |
| 8'-0" | 5973 | 4584 | 1730 | 760 | -534 | 9797 | 10 1/2" | 6" | " | 17683 | 690 | 27.6 |
| 9'-0" | 8505 | 5864 | 2856 | 1560 | -463 | 12809 | 11 1/2" | 6 1/2" | odd 3 #5 | 14521 | 675 | 30.1 |
| | 11667 | 7211 | 4388 | 3167 | 55 | 15710 | 11 1/2" | 6 1/2" | " | 17810 | 828 | 35.3 |
| | 5528 | 8608 | 6387 | 9842 | -494 | 14295 | 11 1/2" | 6 1/2" | " | 16205 | 802 | 41.0 |

ALTERNATE CONDITION

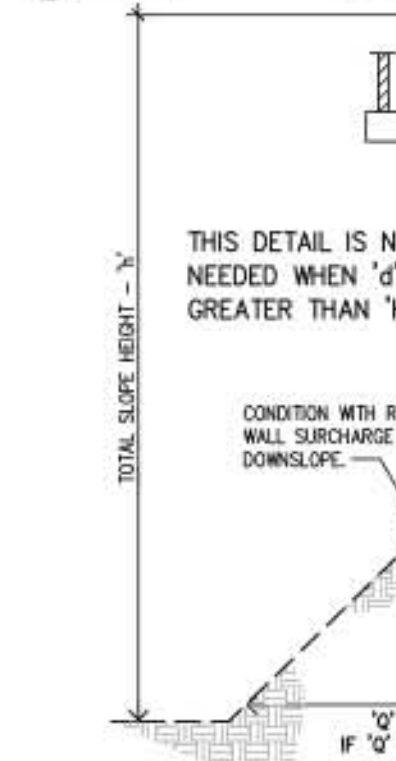
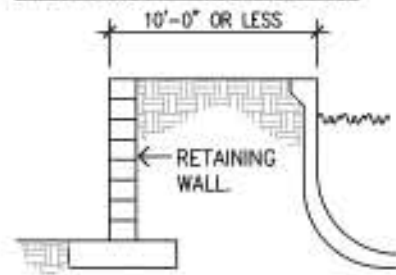


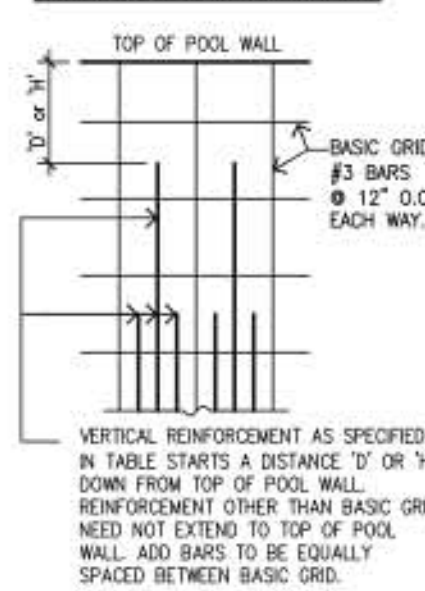
TABLE 540-70

'D' OR 'H' IS DISTANCE FROM TOP OF POOL WALL DOWNWARD. BEGIN SPECIFIED STEEL & GUNITE THICKNESS AT INDICATED 'D' OR 'H' DEPTH (SEE STANDARD STRUCTURAL PLAN, DETAIL #2)

| POOL DEPTH | NO RAISED BOND BEAM | | | REQ'D TRANS. |
|------------|---------------------|--------|----------------|--------------|
| D | d1 | d2 | VERTICAL STEEL | |
| 0 to 1'6" | 3" | 3" | #3 @ 12" | 2'-0" |
| 2'-0" | 3" | 3" | #3 @ 6" | 2'-0" |
| 2'-6" | 3" | 3" | " | 2'-0" |
| 3'-0" | 3 1/2" | 3" | " | 2'-0" |
| 3'-6" | 4 1/2" | 3" | " | 2'-0" |
| 4'-0" | 5" | 3" | #3 @ 3" | 2'-3" |
| 4'-6" | 6" | 3" | " | 3'-0" |
| 5'-0" | 7" | 3" | " | 3'-4" |
| 5'-6" | 8" | 3 1/2" | " | 3'-5" |
| 6'-0" | 8" | 3 1/2" | odd 3 #4 | 3'-9" |
| 6'-6" | 8" | 4" | " | 3'-11" |
| 7'-0" | 8 1/2" | 5" | " | 3'-10" |
| 7'-6" | 9 1/2" | 5 1/2" | " | 3'-8" |
| 8'-0" | 10 1/2" | 6" | " | 3'-6" |
| 8'-6" | 11 1/2" | 6 1/2" | " | 3'-4" |

| TOTAL HEIGHT | 2'-6" MAX. RAISED BOND BEAM | | | REQ'D TRANS. |
|--------------|-----------------------------|--------|----------------|--------------|
| H | d1 | d2 | VERTICAL STEEL | |
| 0 to 1'6" | 3" | 3" | #3 @ 12" | 2'-0" |
| 2'-0" | 3" | 3" | #3 @ 6" | 2'-0" |
| 2'-6" | 3" | 3" | " | 2'-0" |
| 3'-0" | 3 1/2" | 3" | " | 2'-0" |
| 3'-6" | 4 1/2" | 3" | " | 2'-0" |
| 4'-0" | 5" | 3" | #3 @ 3" | 2'-4" |
| 4'-6" | 6" | 3" | " | 3'-0" |
| 5'-0" | 7" | 3" | " | 3'-9" |
| 5'-6" | 8" | 3 1/2" | " | 4'-3" |
| 6'-0" | 8" | 3 1/2" | odd 3 #4 | 4'-10" |
| 6'-6" | 8" | 4" | " | 5'-4" |
| 7'-0" | 8 1/2" | 5" | " | 5'-6" |
| 7'-6" | 9 1/2" | 5 1/2" | " | 5'-4" |
| 8'-0" | 10 1/2" | 6" | " | 5'-2" |
| 8'-6" | 11 1/2" | 6 1/2" | " | 4'-11" |
| 9'-0" | 11 1/2" | 6 1/2" | odd 3 #5 | 4'-10" |
| 9'-6" | 11 1/2" | 6 1/2" | " | 4'-10" |
| 10'-0" | 11 1/2" | 6 1/2" | " | 4'-10" |
| 11'-0" | 11 1/2" | 6 1/2" | " | 4'-10" |

TYPICAL ADD BAR REINFORCING DIAGRAM



THIS DETAIL TO BE USED WHEN FREESTANDING CONDITION IS REQUIRED IN CONJUNCTION WITH RETAINING WALL SURCHARGE.

NOTE: THIS DETAIL IS NOT SUITABLE FOR USE WITH INVERTED FOOTING RETAINING WALLS (NO TOE). RETAINING WALL BY OTHERS OR EXISTING RETAINING WALL.

BEGINNING @ D = 5'-0" USE #3 BARS @ 12" O.C.
 BEGINNING @ D = 6'-0" OR MORE USE #4 BARS @ 12" O.C.

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Date: 3/15/2021

21-03789

FREESTANDING WALL
 WITH RETAINING WALL SURCHARGE
 MAX. FOOTING BEARING PRESSURE = 1,500 P.S.F.
 EQUIVALENT FLUID PRESSURE = 70 P.C.F.

DETAIL #540-70

PLAN VALID ONLY WITH ENGINEER'S SIGNATURE IN RED INK ON PLAN.
 THIS DETAIL TO BE USED IN CONJUNCTION WITH STANDARD POOL STRUCTURAL PLAN

CALCULATIONS

METHODOLOGY:

γ = EQUIVALENT FLUID PRESSURE

CASE I
 $OTM = 1/6 \gamma H^2$ WHERE $\gamma = 70$ pcf
 NET MOM = OTM - RESISTING MOMENT

CASE II
 $OTM = 1/6 \gamma H^2$ WHERE $\gamma = 62.4$ pcf
 NET MOM = OTM + RESISTING MOMENT

$$f_s = \frac{M(12 \text{ in/ft})}{A_s j d} = \frac{M_t (12)}{A_s (0.887) d}$$

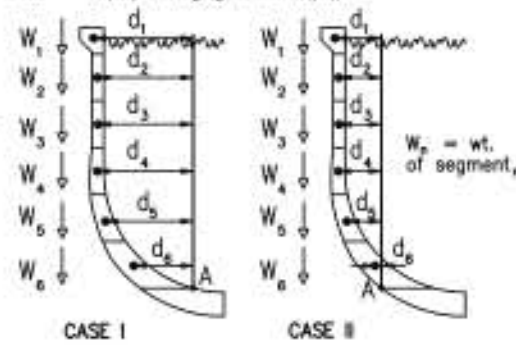
$$f_c = \frac{M(2) 12 \text{ in/ft}}{j k b d^2} = \frac{M_t (2)(12)}{(0.887)(0.339)(12) d^2} < 1125 \text{ psi}$$

$$v_c = \frac{(1/2) \gamma H^2}{(12 \text{ in/ft}) j d} = \frac{\gamma H^2}{(2)(12)(0.887) d} < 55 \text{ psi}$$

$f'_c = 2,500$ psi
 $F_s = 20,000$ psi
 $f_c = 0.45 f'_c = 1125$ psi
 $v_c = 1.1 \sqrt{f'_c} = 55$ psi

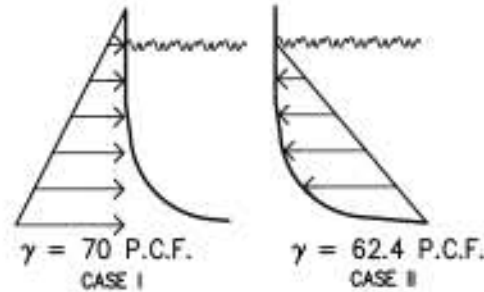
RESISTING MOMENT:

RESISTING MOMENT ABOUT POINT A
 $RM = W_1 d_1 + W_2 d_2 + \dots - W_n d_n$



LOADING DIAGRAM:

THIS DETAIL IS DESIGNED FOR EACH OF THE LOAD CASES DEFINED BELOW.



CALCULATION RESULTS:

FREESTANDING WALL
 EQUIVALENT FLUID PRESSURE = 70 P.C.F.
 RESULTS FOR NO RAISED BOND BEAM

| DEPTH 'D' | SOIL OTM ft-# | WATER OTM ft-# | SOIL RM ft-# | WATER RM ft-# | NET MOM | CASE I d1 SOIL | CASE II d2 WATER | VERTICAL STEEL | f_s p.s.i. | f_c p.s.i. | v_c p.s.i. |
|-----------|---------------|----------------|--------------|---------------|---------|----------------|------------------|----------------|--------------|--------------|--------------|
| 3'-6" | 500 | 446 | 106 | -53 | 460 | 3" | 3" | #3 @ 12" | 18049 | 478 | 11.9 |
| 4'-0" | 747 | 666 | 124 | -55 | 685 | 3" | 3" | #3 @ 6" | 13828 | 547 | 15.6 |
| 5'-0" | 1458 | 1300 | 226 | -14 | 1232 | 3" | 3 1/2" | #3 @ 4" | 16937 | 855 | 24.3 |
| 6'-0" | 2520 | 2246 | 470 | 74 | 2513 | 4" | 4 1/2" | #3 @ 3" | 17164 | 808 | 26.3 |
| 7'-0" | 4002 | 3567 | 982 | 289 | 4039 | 5" | 5 1/2" | add 3 #4 | 14452 | 795 | 28.6 |
| 8'-0" | 5973 | 5325 | 2168 | 771 | 6095 | 5" | 7" | " | 16916 | 880 | 37.3 |
| 8'-6" | 7165 | 6387 | 4813 | 474 | 6861 | 5" | 7 1/2" | " | 17708 | 807 | 42.1 |

RESULTS FOR 2'-6" MAX. RAISED BOND BEAM

| HEIGHT 'H' | SOIL OTM ft-# | WATER OTM ft-# | SOIL RM ft-# | WATER RM ft-# | NET MOM | CASE I d1 SOIL | CASE II d2 WATER | VERTICAL STEEL | f_s p.s.i. | f_c p.s.i. | v_c p.s.i. |
|------------|---------------|----------------|--------------|---------------|---------|----------------|------------------|----------------|--------------|--------------|--------------|
| 3'-6" | 500 | 10 | 106 | -53 | 394 | 3" | 3" | #3 @ 12" | 15461 | 410 | 11.9 |
| 4'-6" | 1063 | 83 | 130 | -105 | 933 | 4" | 3" | #3 @ 6" | 13954 | 466 | 14.8 |
| 5'-6" | 1941 | 281 | 156 | -131 | 1785 | 4" | 3" | #3 @ 4" | 18133 | 769 | 22.1 |
| 6'-6" | 3204 | 666 | 203 | -235 | 3001 | 6" | 3" | " | 19944 | 666 | 20.5 |
| 7'-0" | 4002 | 948 | 271 | -223 | 3731 | 6" | 3" | #3 @ 3" | 18839 | 745 | 23.8 |
| 8'-0" | 5973 | 1730 | 550 | -171 | 5424 | 7 1/2" | 3 1/2" | add 3 #4 | 13999 | 758 | 24.9 |
| 9'-0" | 8505 | 2856 | 1129 | 62 | 7376 | 7 1/2" | 4 1/2" | " | 19037 | 879 | 31.5 |
| 9'-6" | 10003 | 3567 | 1604 | 284 | 8399 | 8" | 5" | add 3 #5 | 14032 | 842 | 32.9 |
| 10'-6" | 13506 | 5325 | 3374 | 924 | 10131 | 8 1/2" | 6 1/2" | " | 15876 | 849 | 37.8 |
| 11'-0" | 15528 | 6387 | 7219 | 349 | 6735 | 8 1/2" | 6 1/2" | " | 14017 | 886 | 41.5 |

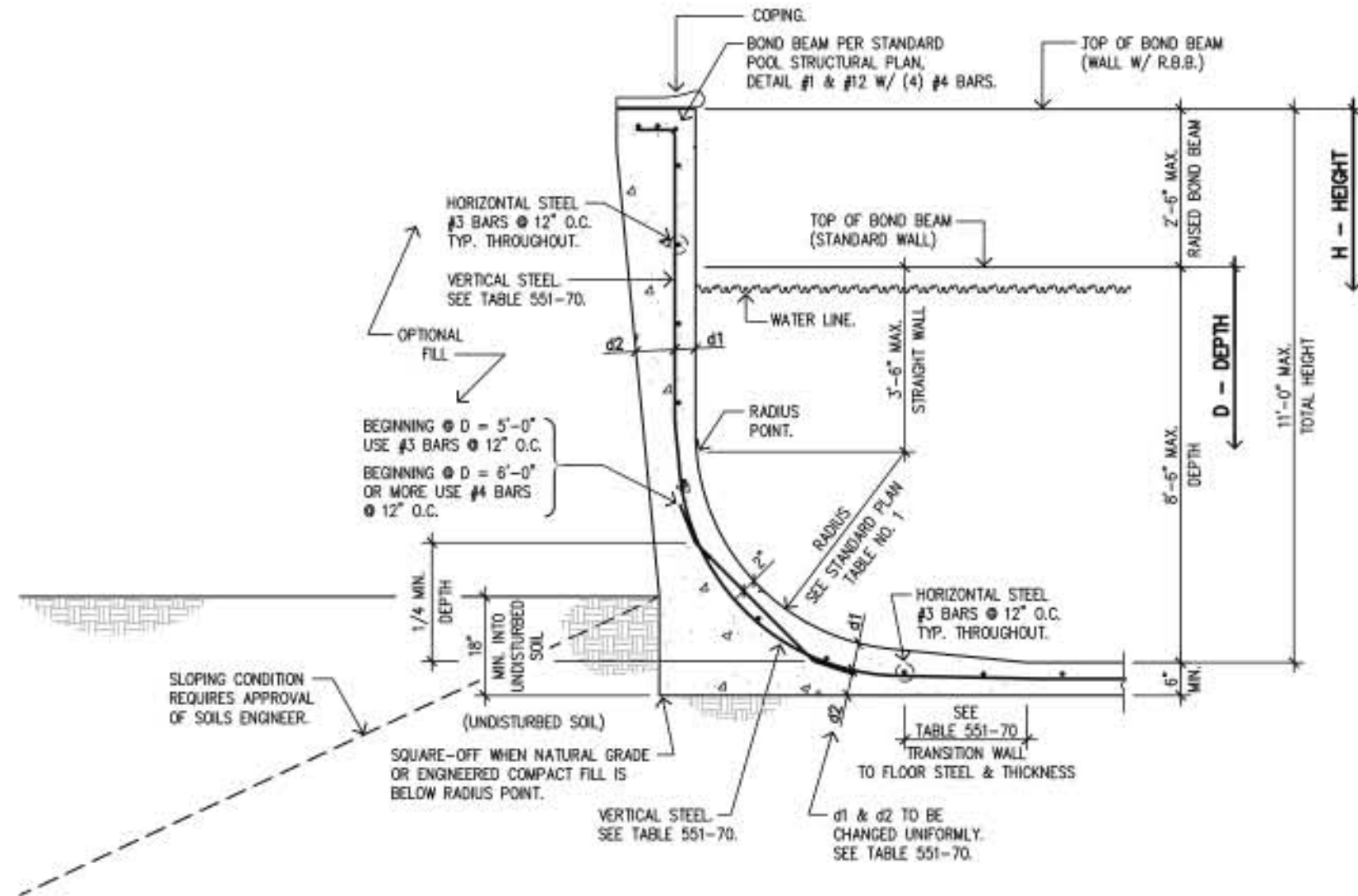


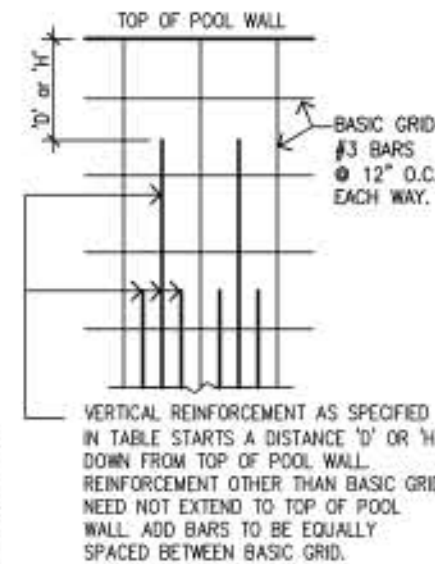
TABLE 551-70

'D' OR 'H' IS DISTANCE FROM TOP OF POOL WALL DOWNWARD.
 BEGIN SPECIFIED STEEL & GUNITE THICKNESS AT INDICATED 'D' OR 'H' DEPTH.
 (SEE STANDARD STRUCTURAL PLAN, DETAIL #2)

| POOL DEPTH | NO RAISED BOND BEAM | | | REQ'D TRANS. |
|------------|---------------------|--------|----------------|--------------|
| D | d1 | d2 | VERTICAL STEEL | |
| 0 to 2'6" | 3" | 3" | #3 @ 12" | 2'-0" |
| 3'-0" | 3" | 3" | #3 @ 6" | 2'-0" |
| 4'-0" | 3" | 3" | #3 @ 4" | 2'-0" |
| 4'-6" | 3" | 3" | " | 2'-0" |
| 5'-0" | 3" | 3 1/2" | #3 @ 3" | 2'-0" |
| 6'-0" | 4" | 4 1/2" | add 3 #4 | 2'-0" |
| 7'-0" | 5" | 5 1/2" | " | 2'-0" |
| 7'-6" | 5" | 6" | " | 2'-0" |
| 8'-0" | 5" | 7" | " | 2'-0" |
| 8'-6" | 5" | 7 1/2" | " | 2'-0" |

| TOTAL HEIGHT | 2'-6" MAX. RAISED BOND BEAM | | | REQ'D TRANS. |
|--------------|-----------------------------|--------|----------------|--------------|
| H | d1 | d2 | VERTICAL STEEL | |
| 0 to 3'0" | 3" | 3" | #3 @ 12" | 2'-0" |
| 3'-6" | 3" | 3" | #3 @ 6" | 2'-0" |
| 4'-0" | 4" | 3" | " | 2'-0" |
| 4'-6" | 4" | 3" | #3 @ 4" | 2'-0" |
| 5'-6" | 4" | 3" | " | 2'-0" |
| 6'-0" | 5" | 3" | #3 @ 3" | 2'-2" |
| 6'-6" | 6" | 3" | " | 2'-8" |
| 7'-0" | 6" | 3" | add 3 #4 | 3'-2" |
| 7'-6" | 7" | 3" | " | 3'-3" |
| 8'-0" | 7 1/2" | 3 1/2" | " | 3'-3" |
| 8'-6" | 7 1/2" | 4" | add 3 #5 | 3'-4" |
| 9'-0" | 7 1/2" | 4 1/2" | " | 3'-5" |
| 10'-0" | 8 1/2" | 5 1/2" | " | 3'-8" |
| 10'-6" | 8 1/2" | 6 1/2" | " | 3'-8" |
| 11'-0" | 8 1/2" | 6 1/2" | " | 3'-9" |

TYPICAL ADD BAR REINFORCING DIAGRAM



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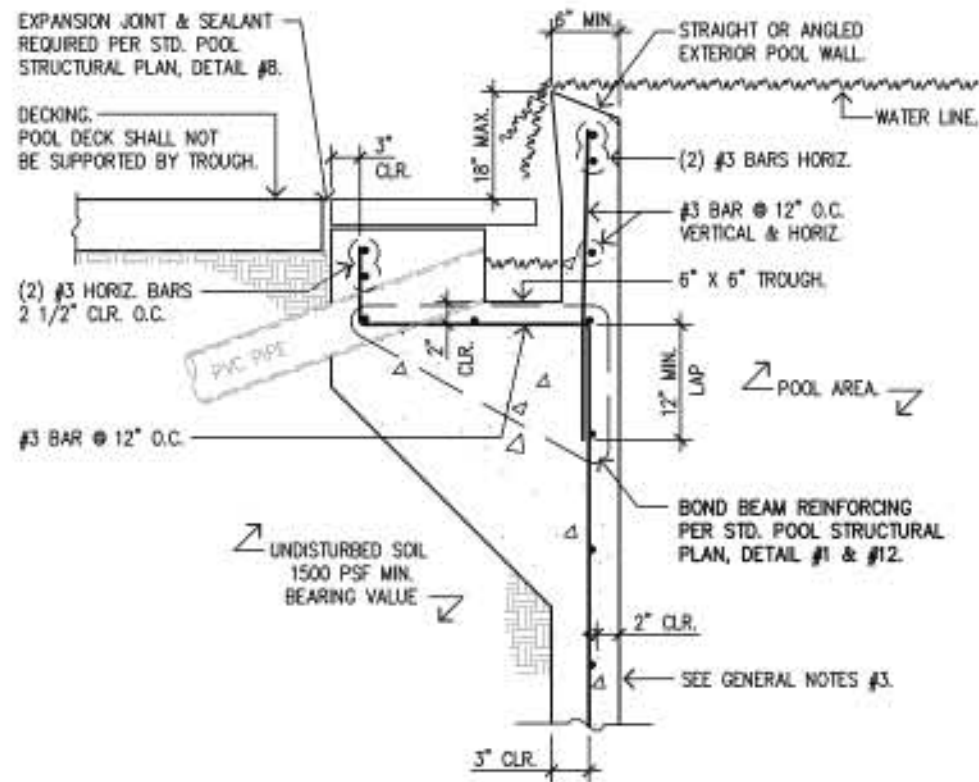
FREESTANDING WALL
 EQUIVALENT FLUID PRESSURE = 70 P.C.F.

DETAIL #551-70

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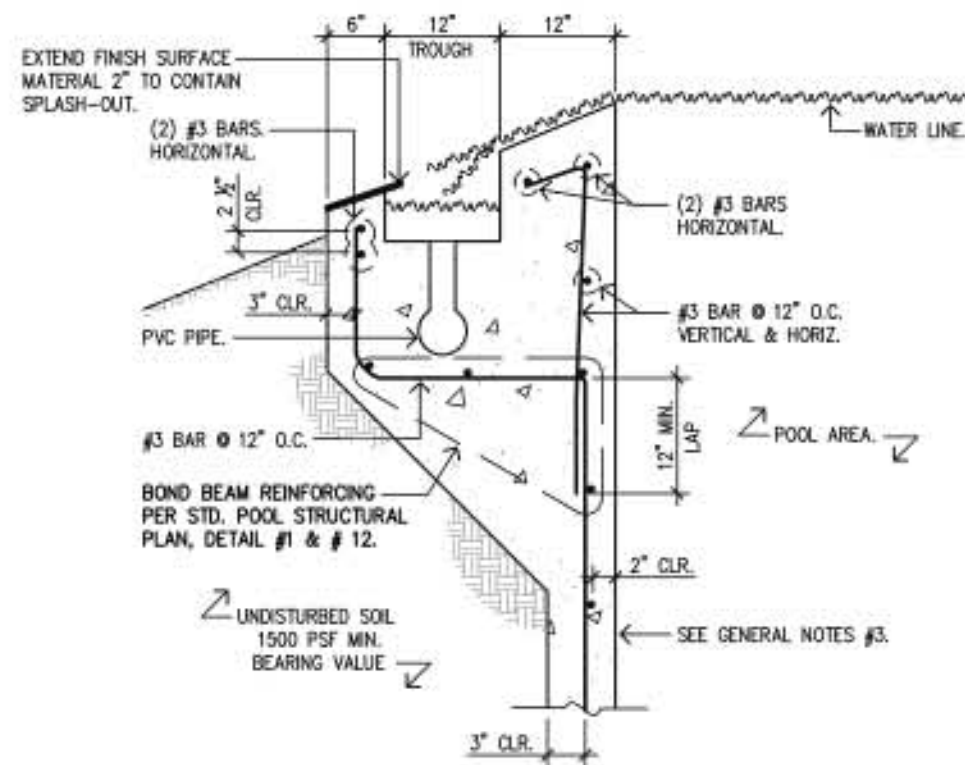


PLAN VALID ONLY WITH ENGINEER'S SIGNATURE IN RED INK ON PLAN.
 THIS DETAIL TO BE USED IN CONJUNCTION WITH STANDARD POOL STRUCTURAL PLAN



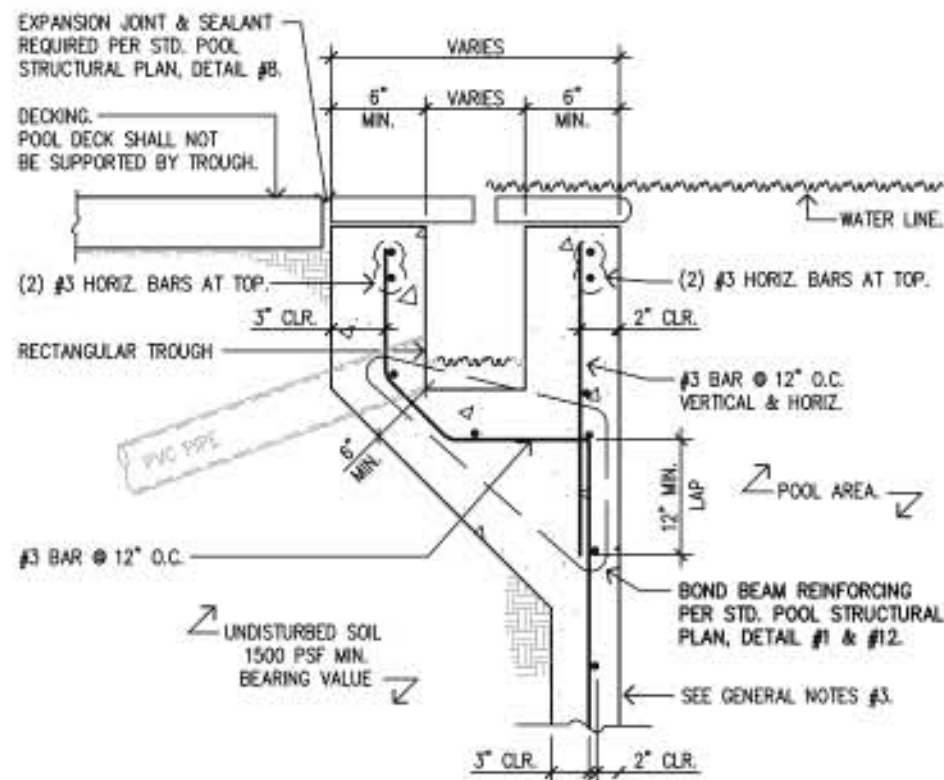
TROUGH AT LEVEL EDGE

A



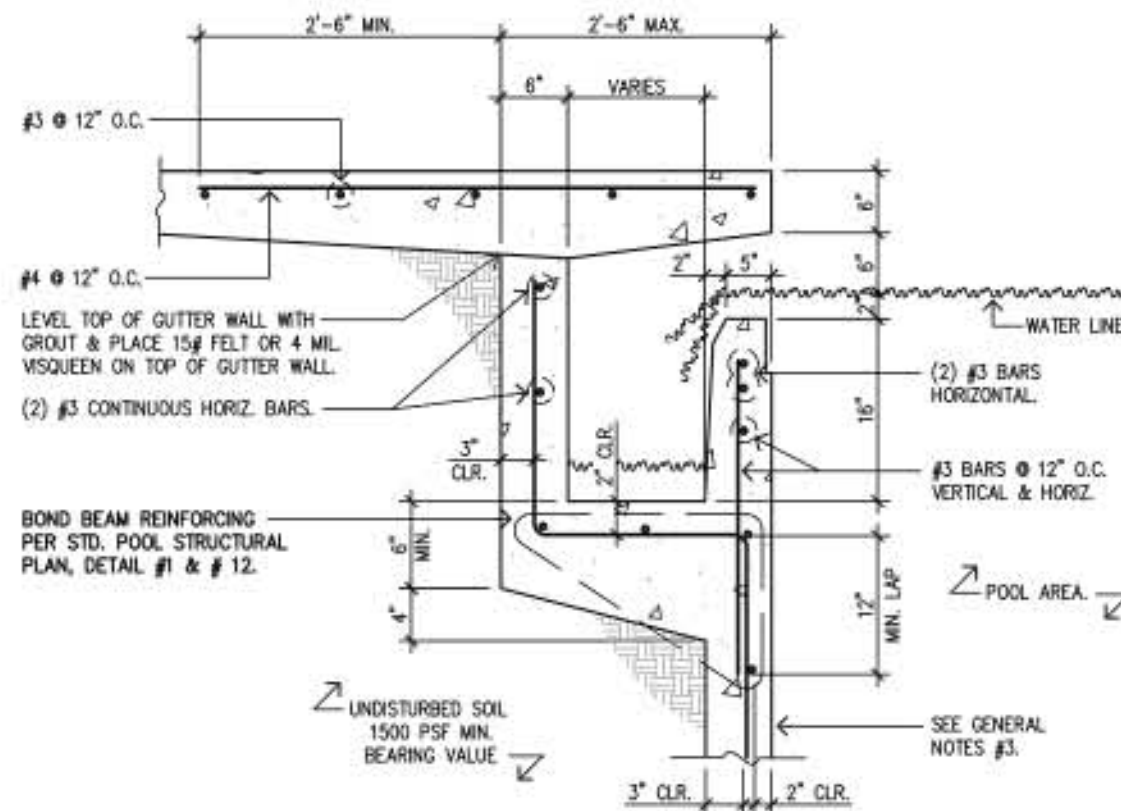
TROUGH AT TOP OF WEIR WALL

B



TROUGH AT ZERO EDGE

C



OVERFLOW GUTTER

D

GENERAL NOTES:

1. LAP ALL STEEL 24" MIN. UNLESS NOTED OTHERWISE.
2. MAINTAIN 2" CLEARANCE BETWEEN ALL PLUMBING & REINFORCING STEEL.
3. FOR POOL WALL GUNITE THICKNESS & STEEL REINFORCING, SEE STANDARD POOL STRUCTURAL PLAN.
4. FINAL CONFIGURATION OF WEIR AS DIRECTED BY CONTRACTOR.
5. GUTTER TROUGH TO BE WATERPROOFED.
6. SPECIAL PRECAUTIONS MUST BE TAKEN TO WATERPROOF PENETRATIONS INTO TROUGH.

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PLAN VALID ONLY WITH ENGINEER'S SIGNATURE IN RED INK ON PLAN.
THIS DETAIL TO BE USED IN CONJUNCTION WITH STANDARD POOL STRUCTURAL PLAN

VANISHING OR ZERO EDGE TROUGH & OVERFLOW GUTTER DETAIL

DETAIL #649